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FREDERICK P. CLARK ASSOCIATES, INC.

PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FAIRFIELD, CONNECTICUT

TRAFFIC ACCESS & IMPACT STUDY

Residential Development 147 Daniels Farm Road Trumbull, Connecticut





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Residential Development 147 Daniels Farm Road Trumbull, Connecticut

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FREDERICK P. CLARK ASSOCIATES, INC.

PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FAIRFIELD, CONNECTICUT

October 17, 2017

Daniels Farm Estates, LLC c/o Stephen R. Bellis 475 Whitney Avenue New Haven, Connecticut 06511

Subject

Traffic Study – Proposed Residential Development – 147 Daniels Farm Road, Trumbull, Connecticut

Dear Mr. Bellis:

We are pleased to submit this Traffic Impact & Access Study for the proposed residential development comprising 32 single-family homes. The parcel of land is located along the westerly side of Daniels Farm Road between Vista Place and Ward Place.

The following sections describe existing conditions on adjacent roadways, current and future traffic volumes, site traffic generation, site access considerations, accident history and the potential impacts to Daniels Farm Road in the immediate vicinity of the subject property frontage.

Project Description

The proposal is to remove one single-family home and construct a private access roadway to serve 32 single-family homes. It is located on the westerly side of Daniels Farm Road in Trumbull, Connecticut. Each home will have an individual driveway and garage to the new internal roadway. For the purposes of completing this traffic analysis it is assumed the development will be approved, constructed and fully occupied by the end of 2019.

Roadways

Daniels Farm Road is a north-south, two-lane, State-maintained, roadway providing access to the site. It is designated State Route 734 in the vicinity of the site. The roadway begins to the south at the signalized intersection of Church Hill Road and White Plains Road, both which are designated as State Route 127. It continues in a northerly direction past the subject property and over State Route 25 to the north.

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Daniels Farm Road continues as a north-south, two-lane, Town-maintained roadway north of the State Route 25 northbound on-and off-ramps.

Along the site frontage Daniels Farm Road provides one travel lane in each direction, a double yellow centerline and white shoulder lines. The travel lanes measure approximately 12-feet, plus provide shoulders of one foot. There are sidewalks along the easterly side of the road for the entire segment of Daniels Farm Road. The posted speed limit is 30 miles per hour on this roadway in the vicinity of the site. Southwest of the site frontage there is a sharp curve on Daniels Farm Road, with a posted warning reduce speed sign of 20 miles per hour and flashing warning lights.

Ward Place, located immediately south of the subject, serves a residential neighborhood. It includes asphalt curbs and absent of any pavement markings or parking restrictions. The road is controlled by a STOP sign on its approach to Daniels Farm Road

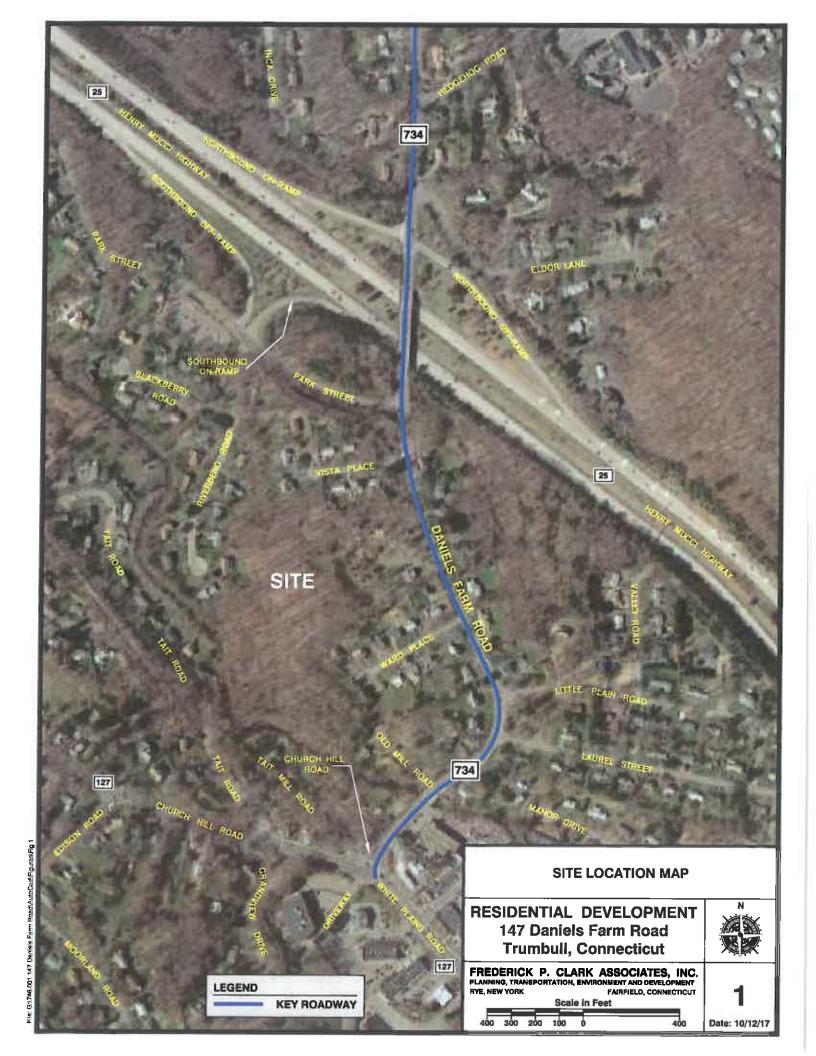
Vista Place, located immediately north of the subject property, is very similar in character to Ward Place. It terminates in a cul-de-sac and serves a residential neighborhood. There are no curbs or pavement markings along this roadway. The road is controlled by a STOP sign at its approach to Daniels Farm Road.

Figure 1 provides a graphic illustration of the site's location in relation to the Study Area roadways. Figure 2 provides the current street system characteristics in the vicinity of the site.

Traffic Volumes

To identify current traffic volumes on Daniels Farm Road representatives of Frederick P. Clark Associates, Inc. installed an automatic traffic recorder (ATR) on this road in the immediate vicinity of the subject property frontage, just south of Vista Place. This ATR was installed for a period beginning Monday, September 25 through Monday, October 2, 2017. The counts were conducted when Schools were open and during appropriate weather conditions.

The ATR identified the daily traffic volume for a typical day, and specifically Thursday, September 28, 2017 as 11,265 vehicles. The northbound traffic flow was found to be 5,873 and the southbound traffic flow was 5,392 vehicles.





STOP SIGN

TRAFFIC SIGN

00 MPH SPEED LIMIT

RESIDENTIAL DEVELOPMENT 147 Daniels Farm Road Trumbull, Connecticut



FREDERICK P. CLARK ASSOCIATES, INC.
PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT
RYE, NEW YORK

FAIRFIELD, CONNECTICUT

			Sca	le in Feet	
200	150	100	50	ò	200

2Date: 10/12/17

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This data indicates that the weekday morning peak hour occurred between 7:00 and 8:00 A.M., with a two-way volume of 775 vehicles. The northbound traffic flow was found to be 392 vehicles and the southbound traffic flow was 383 vehicles. The weekday afternoon peak hour was identified as 5:00 to 6:00 P.M., with 593 vehicles traveling northbound and 503 vehicles traveling southbound for a total of 1,096 vehicles.

Figure 3 graphically illustrates the hourly and directional traffic volumes on Daniels Farm Road along the site frontage. It identifies a weekday morning commuter peak hour and again, although at a higher volume, the weekday afternoon peak hour. Throughout the day the volumes are approximately 5 to 50 percent lower between 9:00 A.M. and 4:00 P.M.

Based on the results of the traffic counting program, the peak hour volumes were identified, as noted above, and graphically illustrated in Figure 4 for the weekday morning and weekday afternoon peak hours. The field sheets and raw data are included in the Appendix of this report.

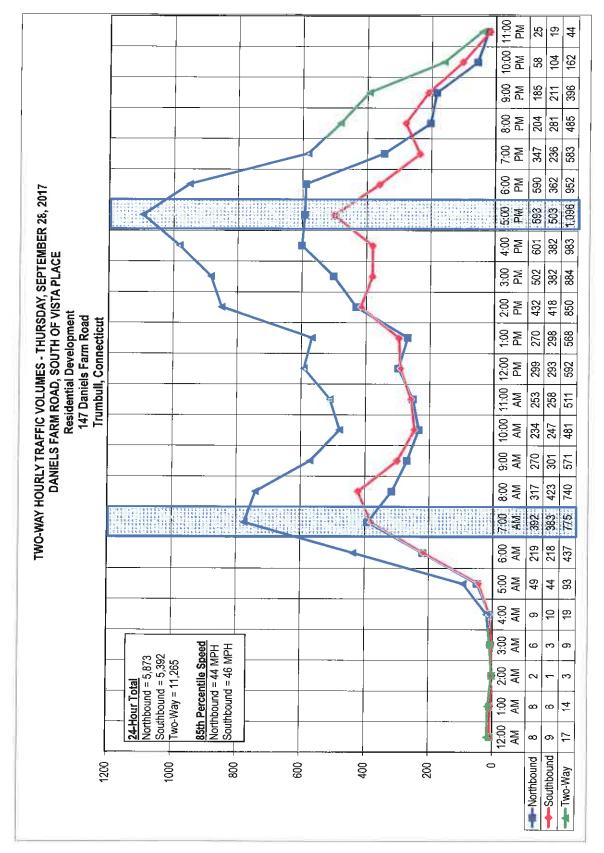
Accident Experience

Accident data was obtained from ConnDOT for a period beginning January 1, 2014 through December 31, 2014 and from the UCONN Crash Data Repository (a new format and process) for a period beginning January 1, 2015 through December 31, 2016 for Daniels Farm Road (State Route 734) in the immediate vicinity of the subject property frontage. Overall, accident data was obtained for a 3-year period.

For Daniels Farm Road (State Route 734), between Ward Place and Vista Place, there was one reported accident during this three-year period. Data indicates that the accident was limited to property damage. The collision type was a rear-end collision with a contributing factor of driver inattention. It was found that the accident occurred during daylight hours and on wet road conditions. Table 1 provides a more detailed summary of the accident data. The accident data obtained from both ConnDOT and the Connecticut Crash Data Repository is included in the Appendix of this report.

2019 No-Build Traffic Volumes

To analyze a future traffic condition, the 2017 traffic volumes described above were expanded to reflect a 2019 no-build condition. An annual growth rate of two percent per year was employed to



Source: Automatic Traffic Recorders conducted by Frederick P. Clark Associates, Inc. from Monday, September 25 to Monday, October 2, 2017

Frederick P. Clark Associates, Inc. October 2017

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 A.M.) VISTA PLACE 392 SITE FARMS ROAD WARD PLACE DANIELS **WEEKDAY AFTERNOON PEAK HOUR** (5:00 - 6:00 P.M.) VISTA PLACE 593 SITE FARMS ROAD WARD PLACE DANIELS **2017 EXISTING TRAFFIC VOLUMES** Note: Two-way hourly traffic volumes from RESIDENTIAL DEVELOPMENT **Automatic Traffic Recorders conducted by** 147 Daniels Farm Road Frederick P. Clark Associates, Inc. from Monday, September 25 to Monday, October 2, 2017. Trumbull, Connecticut FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FAIRFIELD, CONNECTICUT Not to Scale Date: 10/12/17

Table 1 ACCIDENT EXPERIENCE SUMMARY – STATE ROUTE 734 Residential Development 147 Daniels Farm Road Trumbull, Connecticut

	STATE F	ROUTE 734
	Between Ward Pl	ace and Vista Place
ACCIDENT CHARACTERISTICS	Total	%
Year		
- 2014	0	0
= 2015	1	100
2 016	0	0
■ Total	1	100
Accident Severity	*10	
Property Damage	1	100
Injury	0	0
Collision Type		,
■ Rear-end	1	100
Contributing Factor		
 Driver Inattention 	1	100
Light Condition		
Daylight	1	100
Surface Condition	,	
■ Wet	1	100
Weather Conditions		
■ Clear	1	100

Source:

- 1) Connecticut Department of Transportation from January 1, 2014 to December 31, 2014
- 2) Connecticut Crash Data Repository from January 1, 2015 to December 31, 2016.

Notes:

- 1) January 1, 2014 to December 31, 2016 is the latest three full years of accident data available.
- 2) The 2015 accident data follows the new Connecticut Uniform Police Report. The collision type was determined using the manner of crash/collision impact and the accident diagram. For weather conditions, no adverse was replaced with clear.

Frederick P. Clark Associates, Inc. G:\746.001 147 Daniels Farm Road\Word\dfr17-001.stc.docx 9/25/17

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2019 to represent the future traffic volumes. The future 2019 projected traffic volumes are graphically illustrated in Figure 5 for the weekday morning and weekday afternoon peak hours.

Based on the discussions with the Town Planning Department, there is one other planned development at 965 White Plains Road which is included in the no-build condition. Traffic for this development is accounted for based on its traffic study. Figure 6 graphically illustrates the other development traffic volumes for the weekday morning and weekday afternoon peak hours.

Future 2019 no-build traffic volumes are graphically illustrated in Figure 7 for the weekday morning and weekday afternoon peak hours and include the projected traffic volumes and other development traffic volumes.

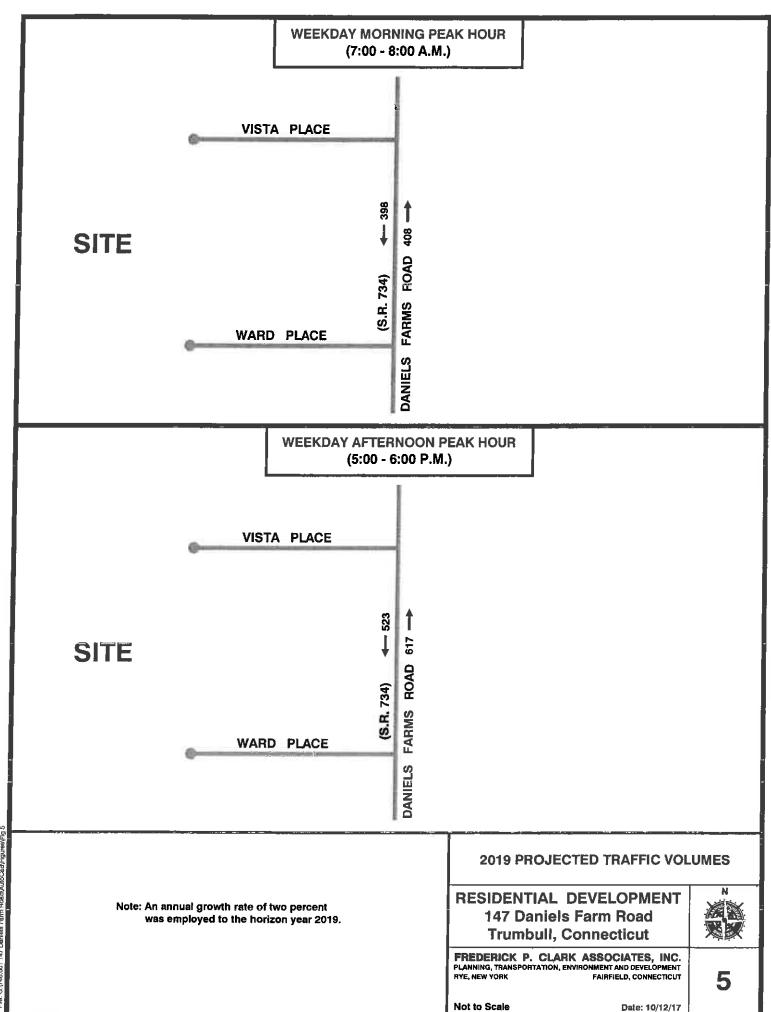
Estimation of Site Traffic Generation

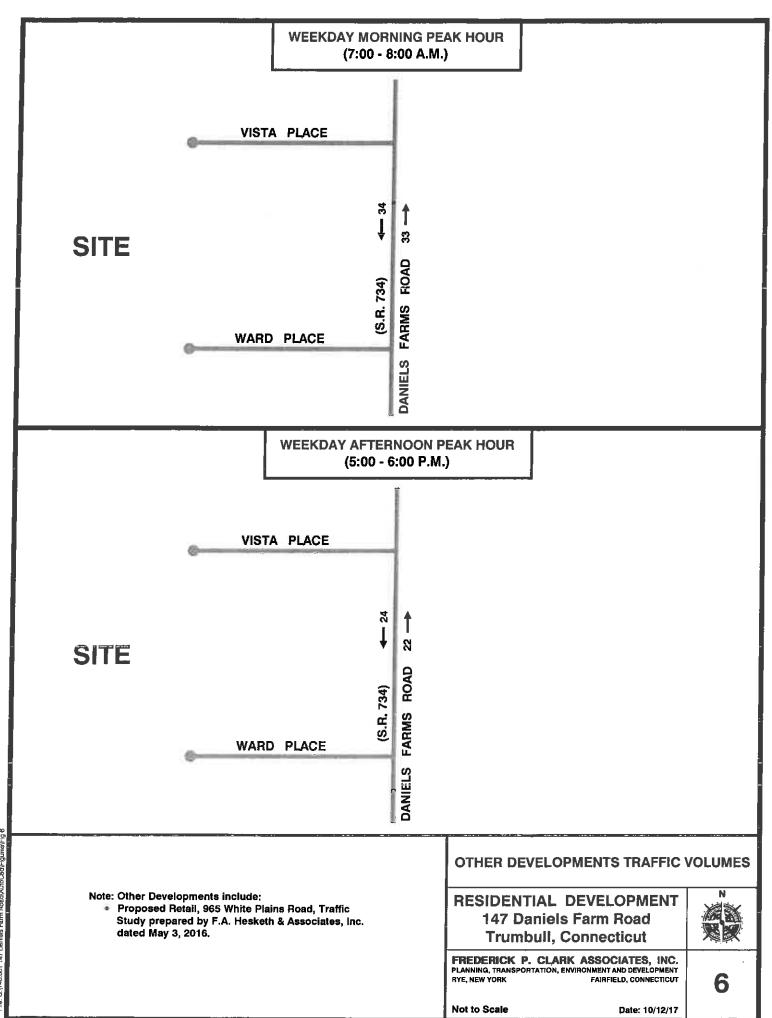
A development of this type and size, which includes 32 single-family homes, is estimated to generate a total of 32 and 38 vehicle trip ends during the weekday morning and weekday afternoon peak hours, respectively. These site traffic estimates are based on trip generation rates provided by the Institute of Transportation Engineers (ITE) and published in "Trip Generation," 9th Edition, 2012. Table 2 provides a more detailed breakdown of entering and exiting traffic patterns for both the weekday morning and weekday afternoon peak hours.

Site Access Considerations

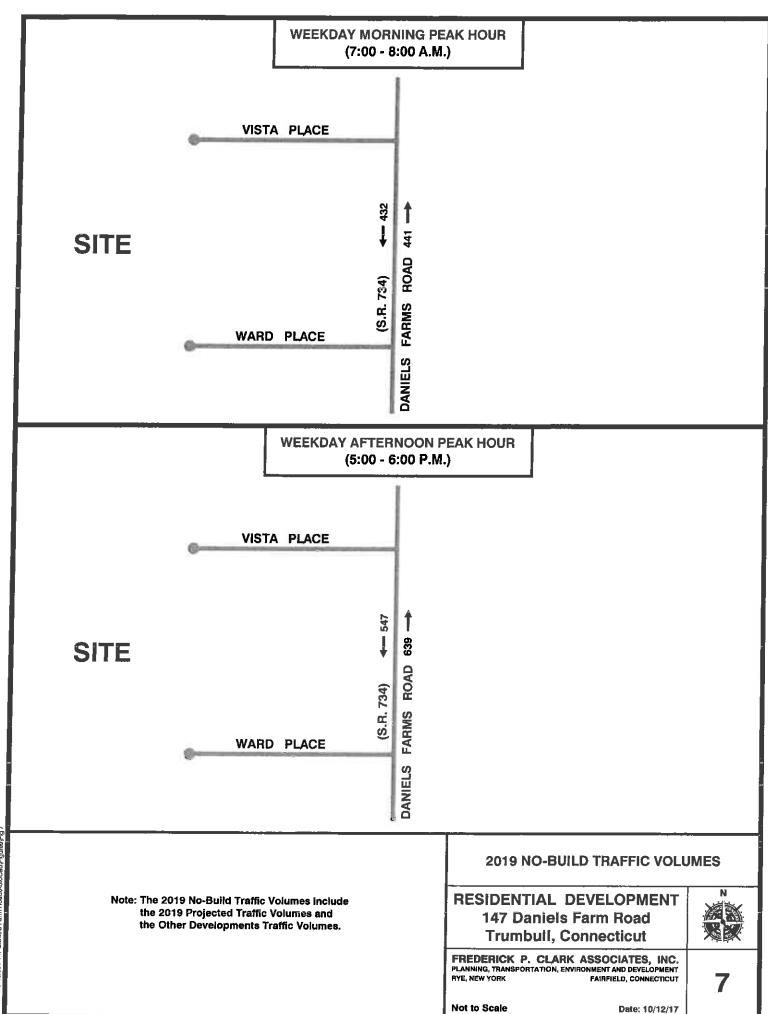
The proposal is to construct an access road from Daniels Farms Road to serve the 32 single-family homes. The new proposed access roadway will function as a two-lane, two-way, roadway and will provide appropriate sight lines in both directions.

As part of the traffic counting program and the installation of the ATR, as noted above, the 85th percentile speed of motorists traveling on this road was recorded for over seven days. Based on the results of this speed analysis it was determined that the 85th percentile speed of motorists traveling in the northbound and southbound directions were 44 and 46 miles per hour, respectively. Although the posted speed limit is 30 miles per hour the higher speed is used in this analysis to be conservative and to address the methodology used by ConnDOT to provide the greatest intersection sight distance, when appropriate and available.





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Table 2 SITE TRAFFIC GENERATION – PEAK HOURS

Residential Development 147 Daniels Farm Road Trumbull, Connecticut

			VEHICLE T	RIP ENDS
LAND USE	SIZE	TRAFFIÇ DIRECTION	Weekday Morning	Weekday Afternoon
Single-Family Detached Housing	32 Dwelling Units	Enter Exit Total	8 <u>24</u> 32	24 <u>14</u> 38

Source: "Trip Generation," 9th Edition, published by the Institute of Transportation Engineers (ITE), 2012 using Single-Family Detached Housing, Code #210 Fitted Curve Equation.

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For the 46 miles per hour speed noted above, an intersection sight distance (ISD) of 512 feet is needed in both directions. Based on the Line of Sight Plan and Profile prepared by Spath-Bjorklund Associates, Inc. dated September 27, 2017, the available ISD is 512 feet, which meets the ConnDOT required ISD for the 85th percentile operating speed. Table 3 provides a more detailed summary of the analysis completed for intersection sight distance at the location of the proposed access roadway.

Distribution and Assignment of Site-Generated Traffic

Based on an evaluation of current traffic patterns on Daniels Farm Road and knowledge of the area, distribution patterns were developed for both the weekday morning and weekday afternoon peak hours.

During the weekday morning peak hour, it is estimated that 50 percent of the site generated traffic will arrive from and depart to both the north and south on Daniels Farm Road. During the weekday afternoon peak hour, it is estimated that 46 percent of the site generated traffic will arrive from and depart to the north on Daniels Farm Road and the remaining 54 percent will arrive from and depart to the south. This distribution and assignment for site traffic is graphically illustrated in Figure 8 for the weekday morning and weekday afternoon peak hours. Figure 9 graphically illustrates the site traffic generation and assignment based on the site traffic the distribution patterns described above for the weekday morning and weekday afternoon peak hours.

2019 Build Traffic Volumes

Figure 10 illustrates the build traffic volumes for the weekday morning and weekday afternoon peak hours, which include the site traffic generation added to the no-build traffic volumes, both which were described previously in this report.

Capacity Analysis Results

Results of the analysis indicate that movements from the two-way, STOP sign controlled approach of the proposed access roadway at Daniels Farm Road, will operate at Level of Service "C" or better during both the weekday morning and weekday afternoon peak hours. The northbound left turn movement will operate at a Level of Service "A" during both peak hours.

INTERSECTION SIGHT DISTANCE (ISD) ANALYSIS Residential Development 147 Daniels Farm Road Trumbull, Connecticut Table 3

|--|

Connecticut Department of Transportation Highway Design Manual 2003 Edition, Revised January 2011, Section 11-2.03.01 Figure 11-2C and Section 11-2.03.02. Source:

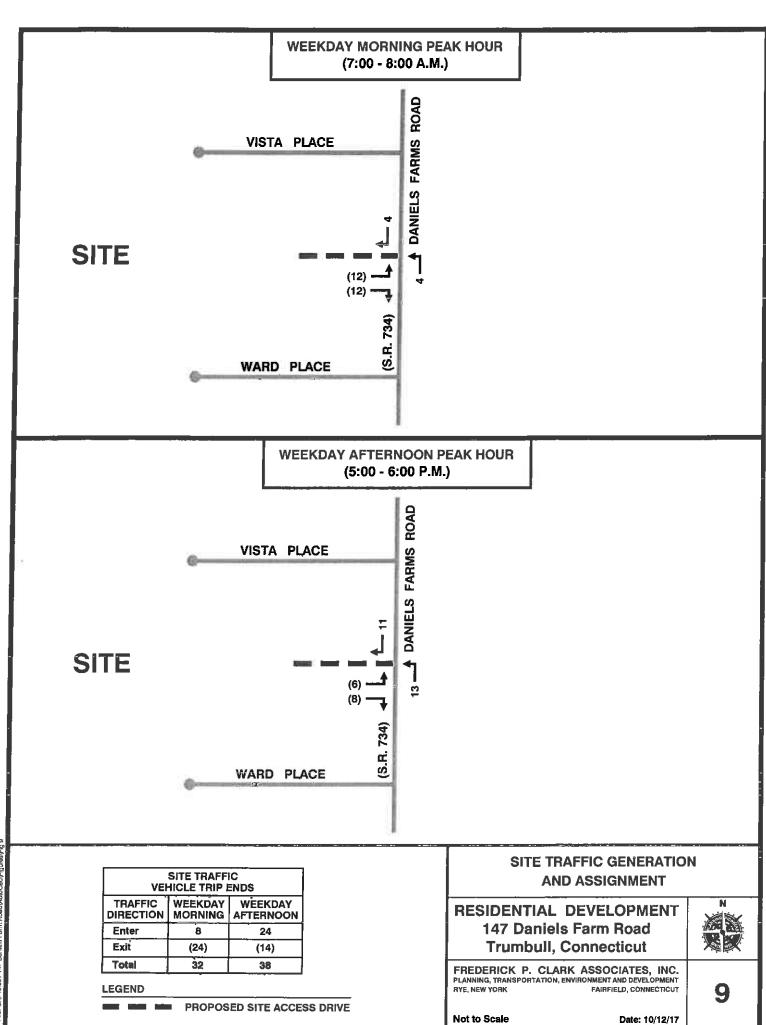
Notes:

- The posted speed limit is 30 miles per hour on Daniels Farm Road in the vicinity of the site.
- The 85th percentile speed of vehicles was measured to be 44 and 46 mile per hour in the northbound and southbound directions, espectively.

Findings: Based on the Line of Sight Plan and Profile prepared by Spath-Bjorklund Associates, Inc. dated September 27, 2017, the available ISD is 512 feet, which meets the ConnDOT required ISD for the 85th percentile operating speed.

Frederick P. Clark Associates, Inc. G:7746.001 147 Daniels Farm Road/Word\dfr17-003.stc.doc 10/16/17

WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 A.M.) ROAD DANIELS FARMS VISTA PLACE SITE (50%) (50%)(S.R. 734) WARD PLACE **WEEKDAY AFTERNOON PEAK HOUR** (5:00 - 6:00 P.M.) ROAD VISTA PLACE DANIELS FARMS SITE (46%)(54%)(S.R. 734) WARD PLACE SITE TRAFFIC DISTRIBUTION SITE TRAFFIC RESIDENTIAL DEVELOPMENT Enter 00% Exit (00%) 147 Daniels Farm Road Trumbull, Connecticut **LEGEND** FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT RYE, NEW YORK FAIRFIELD, CONNECTICUT PROPOSED SITE ACCESS DRIVE 8 Not to Scale Date: 10/12/17



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WEEKDAY MORNING PEAK HOUR (7:00 - 8:00 A.M.) DANIELS FARMS ROAD VISTA PLACE SITE 12 œ છું WARD PLACE **WEEKDAY AFTERNOON PEAK HOUR** (5:00 - 6:00 P.M.) DANIELS FARMS ROAD VISTA PLACE SITE 13 639 WARD PLACE **2019 BUILD TRAFFIC VOLUMES** Note: The 2019 Build Traffic Volumes include RESIDENTIAL DEVELOPMENT the 2019 No-Build Traffic Volumes and the Site Traffic Generation. 147 Daniels Farm Road Trumbull, Connecticut LEGEND PROPOSED SITE ACCESS DRIVE FREDERICK P. CLARK ASSOCIATES, INC. PLANNING, TRANSPORTATION, ENVIRONMENT AND DEVELOPMENT
RYE, NEW YORK
FAIRFIELD, CONNECTICUT 10

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Date: 10/12/17

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Table 4 provide a more detailed summary of the results of the analyses, as described above. This table provide Level of Service, average vehicle delay and volume to capacity ratio for lane and movement during each of the peak hours for a build condition. Table 5 provides a more detailed summary of the results of the Storage/Queue analyses for each lane and movement during each of the peak hours for a build condition. The capacity worksheets are included in the Appendix of this report.

Findings

Results of the analysis indicate that Daniels Farm Road, which is a State Highway, carries a two-way peak hour volume of 775 and 1,096 vehicles during the weekday morning and weekday afternoon peak hours, respectively. The traffic patterns indicate that during the morning peak hour the traffic flow is similar in each direction. During the weekday afternoon peak hour, more traffic is traveling northbound on this section of roadway near the subject property.

A development of this type and size, will generate at most 32 and 38 vehicles trip ends during the weekday morning and weekday afternoon peak hours, respectively. This level of additional traffic added to Daniels Farm Road will have an insignificant, if any, impact to the overall operation of this roadway during any time of the day and specifically during peak hours, which carry the highest volumes. Results of the analysis indicate exiting movements from the proposed access roadway onto Daniels Farm Road will operate at Level of Service "C" during both peak hours. Impact to nearby intersection will be minimal.

The access roadway should include appropriate traffic control, specifically a STOP sign and STOP bar. Clearing of vegetation along the site's frontage should be provided with this proposed residential development.

Sincerely, Mulal 4. John

Michael A. Galante Managing Principal

Enclosure

cc: David Bjorklund, P.E.

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Table 4 CAPACITY ANALYSIS RESULTS FOR UNSIGNALIZED INTERSECTIONS – MEASURE OF EFFECTIVENESS (MOE) – PEAK HOURS

Residential Development 147 Daniels Farm Road Trumbull, Connecticut

			201	19 BUILD (CONDITION	IS
			Weekday	Morning	Weekday	Afternoon
	CONTROL	PHYSICAL	LOS/	V/C	LOS/	V/C
INTERSECTION	TYPE	UNITS	Delay	Ratio	Delay	Ratio
Daniels Farm Road (S.R.	TWSC	EB Ln1	C/15.1	0.068	C/19.1	0.056
734) at Site Access Road		NB L	A/8.3	0.004	A/8.8	0.015

Notes:

- Synchro 10.0/HCM 6th Edition results are used for capacity analysis.
- Level of Service determining parameter is called the service measure.
- TWSC = Two-Way STQP Control.
- For TWSC Intersections: Level of Service/Average Control delay per vehicle (seconds/vehicle).
- V/C ratio indicates the amount of congestion for each Movement and Lane. Any V/C ratio greater than or equal to one indicates that the Movement and Lane are operating at above capacity.
- Physical Units consist of the following:
 - 1. TWSC Intersections: Critical Lane and Critical Movement.

NB = Northbound

EB = Eastbound

SB = Southbound

WB = Westbound

L = Left Turn

T = Through

R = Right Turn

Ln = Lane

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Table 5 STORAGE/QUEUE ANALYSIS RESULTS FOR UNSIGNALIZED INTERSECTIONS – PEAK HOURS Residential Development 147 Daniels Farm Road Trumbull, Connecticut

				2019 BUILD	CONDITIONS
		7		Weekday Morning	Weekday Afternoon
		\$TORAGE/		Queue	Queue
	CONTROL	LINK	PHYSICAL	Length	Length
INTERSECTION	TYPE	LENGTH	ŲNITS	(Veh.)	(Veh.)
Daniels Farm Road (S.R.	TWSC	4.0	EB Ln1	0.2	0.2
734) at Site Access Road		12.0	NB L	0.0	0.0

Notes:

- Synchro 10.0 Macroscopic model/HCM 6th Edition results are used for storage/queue analysis.
- The Queue Length rows show the 95th percentile maximum queue length in vehicles.
- The Queue Length is for each lane. The total queue length is divided by the number of lanes and the lane utilization factor.
- The 95th percentile queue is the maximum back of the queue with the 95th percentile traffic volumes.
- **Bolded** 95th percentile queue exceeds the storage available.
- TWSC = Two-Way STOP Control.
- Physical Units consist of the following:
 - 1. Movement for TWSC Intersections.

L = Left Turn T = Through R = Right Turn Ln = Lane

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APPENDIX

AUTOMATIC TRAFFIC RECORDER

RESIDENTIAL DEVELOPMENT, 147 DANIELS FARM ROAD, TRUMBULL, CT (#746.001) FIELD DATA SUMMARY - DANIELS FARM ROAD, SOUTH OF VISTA PLACE

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Tuesday, September 26, 2017	SB	6	_		4	2	9	37	216	333	393	295	237	212	560	291	463	328	336	412	410	282	127	112	40	16	4 896
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25, 2017	Two-Way		!									261	203	277	289	572	8:1	961	206	666	780	596	385	98	88	8	8.006
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Monday,	æ											5	243	252	333	289	341	200	530	497	443	282	175	6	9	∞	4.190
	TIME	12:00 AM	1:00 AM	2:00 AM	3-00 AM	W. C. C.	4.02.AM	2:00 AM	6:00 AM	7:00 AM	6:00 AM	9:00 AM	10:00 AM	11:00 AM	12:00 PM	1:00 PM	2:00 PM	3:00 PM	4:00 PM	5:00 PM	5.00 PM	33	8:00 PM	906 PM	10:00	11:00 P.W	TOTAL

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| | #17131 DANIELS FARM RD S/O VISTA 1-in-karown 20 mph 25 mph 30 mph 35 mph 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | #17131 DANIELS FARM RD S/O VISTA PL. Unknown 25 mph 30 mph 35 mph 4 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 1 4 24 20 8 45 20 8 45 21 9 68 22 19 68 24 66 2 19 68 2 19 68 3 103 4 1 6 89 4 1 7 88 4 1 88 4 1 88 4 1 88 4 1 98 4 1 98 6 1 1 98 6 1 1 98 7 1 1 6 89 8 6 4 8 6 4 9 6 80 9 7 8 8 88 9 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 | ###################################### | #17131 DANIELS FARM RD S/O VISTA PLACE Unktonom 25 mph 30 mph 35 mph 40 mph 45 mph 50 n 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | #17131 DANIELS FARM RD S/O VISTA PLACE Unknown 25 mph 26 mph 27 mph 28 mph 28 mph 29 mph 20 mp | #17131 DANIELS FARM RD S/O VISTA PLACE Unknown 25 mph 26 mph 27 mph 28 mph 28 mph 29 mph 20 mp | #17131 DANIELS FARM RD S/O VISTA PLACE Distriction 25 mph 26 mph 29 mph 20 mph 2 | #17131 DANIELS FARM RD S/O VISTA PLACE Unknown 255 mph 30 mph 40 mph 45 mph 40 mph 45 mph 55 mph 60 m | #17131 DANIELS FARM RD S/O VISTA PLACE Districtors 25 mph 25 mph 26 mph 27 mph 28 mph 28 mph 28 mph 29 mph 20 mph 2 | The Continue of the Continue | TITI DAMIELS FARM KD S/O VISTA PLACE Information 25 mpth 30 mpth 35 mpth 40 mpth 45 mpth 80 mpth 66 mpth 66 mpth 70 | Latitude: | Started: | | 15 mph | 0 | 0 | 0

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Interval	15 mph	20 mph	25 mph	30 mph	35 mph	40 mph	45 mph	50 mph	55 mph	eo mph	65 mph	70 mph	75 mph	80 mph	> 80 mph	Total
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10 mile pace (entire report duration)					36 - 45	Veh	Vehicles within nace range	Ca range.			CSFOF				2	010/
85th Speed Percentilles (entire report duration)	Ê				!			i i			70404	20	rercentage of total volume:	Volume:		69.13%

Array Type: Tube - Tube,
Deadtime (in ms): 40,
Maximum vehicle length: 110.0 ft,
Maximum inter-axie spacing: 45.0 ft,
Classification Scheme: FHWA-USA,
Sensor Spacing: 6.0 ft,
Report Generated Wednesday, October 04, 2017

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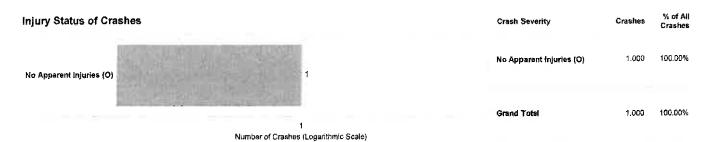
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ACCIDENT HISTORY

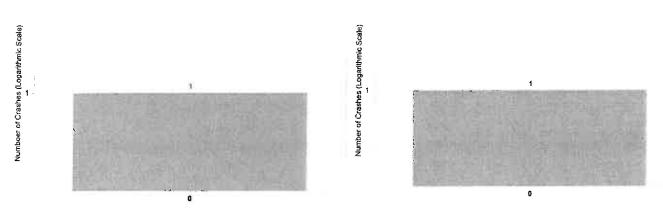
ROUTE	FROM MILE	TO MILE	TOWN NO.		S R I G U	!	R I D N T T Y Y	ACCS	VEHICLES	MILLION VEHICLE MILES	(RAV) AVG. ACC. RATE	(RA) ACT. ACC. RATE	(RC) IMPROB ACC. RATE	ra/rc	LISTED ON SLOSS
734	. 18	.18	144	AT LITTLE PLAIN RD	0.2		2 2	1	12191000	.121910	. 24	.08	.560	. 15	
734	. 19	. 24	144	BET LITTLE PLAIN RD & WARD PL	0.2		2 0	0	12190999	.731460	, 18	.00	.451	.00	
734	. 25	. 25	144	AT WARD PL	0.2		2 2	U	12191000	.121910	. 24	. 00	.560	.00	
734	26	.38	144	BET WARD PL & VISTA PL	0 2	- 8	2 0	0	12190999	1.584830	2.36	.00	5.195	.00	
734	. 39	.39	144	AT VISTA PL	0.2		2 2	٥	12191000	.121910	.24	.00	.560	.00	
734	.40	42	144	BET VISTA PL & SR 739	0 2		20	0	12190999	.365730	. 18	.00	.451	.00	
734	. 43	.43	144	AT SR 739 (PARK ST)	0 2		2 1	1	14034250	.121910	.75	.07	1.307	. 05	
734	. 44	.55	144	BET SR 739 & RT 25 NB RAMPS	0 2		2 0	0	15877499	1.905300	2.36	.00	4.972	.00	
734	.56	.58	144	@ RT 25 NB RAMPS & DANIELS FAR	0 2		21	1	7938750	.476325	. 75	. 13	1.476	. 09	
735	.00	.00	43	AT RT 1 (SALTONSTALL PKWY)	1 2		2 1	6	6989750	.139795	. 75	. 86	1.519	. 56	
735	, 01	.02	43	BET RT 1 & KIMBERLY AVE	0 2		2 0	0	13979499	.279590	.18	.00	.436	.00	armet
735	. 03	.03	43	AT FORBES PL & KIMBERLY AVE	0 2		2 2	36	11150750	.139795	.24	3.23	.573	5.63 .79	YES
735	.04	.20	43	BET FORBES PL & RT 100	0 2		2 0	6	8321999	1.414740	2.36	4.24	5.341 1.720	1.12	
735	. 21	.21	43	AT RT 100 (HIGH ST NORTH)	1 2		2 1	В	41.61000	.083220	. 75 . 75	.32	1.846	.17	
736	.00 .01	.00	83 83	AT RT 162 (NEW HAVEN AVE) BET RT 162 & KNOLLWOOD RD	12		21 20	1	3139000 6277999	.062780 .313900	.18	.16	.535	.30	
736	.06	.05	83	AT KNOLLWOOD RD	02		22	2	6278000	.062780	.24	.32	.664	.48	
736 736	.07	.10	83	BET KNOLLWOOD RD & BUCKINGHAM	0 2		20	0	6277999	.251120	.18	.00	.535	.00	
736	. 11	.11	83	AT BUCKINGHAM PL	0 2		2 2	0	6278000	.062780	. 24	.00	.664	.00	
736	.12	.21	83	BET BUCKINGHAM & ETTADORE PARK	0 2		2 0	1	6277999	.627800	.18	.16	.535	.30	
736	. 22	.22	83	AT ETTADORE PARK	0 2		2 2	ō	6278000	.062780	.24	.00	,664	.00	
736	. 23	.31	83	BET ETTADORE PARK & ETTADORE P	0 2		2 0	ō	6277999	.565020	.18	.00	.535	.00	
736	,32	,32	83	AT ETTADORE PARK	0 2		2 2	1	6278000	.062780	. 24	.16	.664	. 24	
736	.33	.69	В3	BET ETTADORE PARK & OLD FIELD	0 2		2 0	4	6277999	2,322860	2.36	1.72	4.748	.36	
736	.70	.70	83	AT OLD FIELD LA & SHADYSIDE LA	1 2		2 2	2	8212500	.062780	.63	. 24	1,277	.19	
736	.71	. 78	83	BET SHADYSIDE LA & CALF PEN LA	0 2	2	0	1	4671999	.373760	.18	. 21	. 577	.37	
736	.79	.79	83	AT CALF PEN LA	0 2		2	0	4672000	.046720	.24	.00	.717	.00	
736	.80	.92	83	BET CALF PEN LA & TIPPY RD	0 2	2	0 9	0	4671999	.607360	2.36	.00	6.624	.00	
736	. 93	.93	83	AT TIPPY RD	0 2	2	2	0	4672000	.046720	. 24	.00	.717	.00	
736	. 94	. 94	83	BET TIPPY RD & BRYAN HILL RD	0 2	2	2 0	0	4671999	.046720	.18	.00	.577	.00	
736	. 95	.95	83	AT BRYAN HILL RD	0 2	2	2	0	4672000	.046720	.24	.00	.717	.00	
736	. 96	1.03	83	BET BRYAN HILL RD & WELCHS PT	0 2	2	. 0	Ø.	4671999	.373760	.18	.00	.577	.00	
736	1.04	1.04	83	AT WELCHS POINT RD	0.2	2	2	4	3814250	.046720	.24	1.05	.755	1.39	
736	1.05	1.23		BET WELCHS POINT RD & LIBERTY	0.3	2		3	2956499	,561735	2.36	5.34	6.759	.79	
736	1.24	1.24		AT LIBERTY ST	0 2	- 2		0	2956500	.029565	. 24	.00	.805	.00	
736	1.25	1.27		BET LIBERTY ST & CHESTER ST	0.2	- 2		0	2956499	.088695	.18	.00	. 645	.00	
736	1.28	1.28		AT CHESTER ST	0 2	- 2		0	2956500	.029565	. 24	.00	. 805	.00	
736	1.29	1.32		BET CHESTER ST & CLOVER ST	0 2	- 2		0	2956499	.118260	.18	.00	.645	.00	
736	1.33	1.33		AT CLOVER ST	0 2		€2	0	2956500	.029565	.24	. 00	.805	.00	
736	1.34	1.36		BET CLOVER ST & OLIVE ST	0 2	2		0	2956499	.088695	.18	.00	.645	.00	
736	1.37	1.37		AT OLIVE ST	0 2	2		0	2956500	.029565	.24	.00	. 805	.00	
736	1.38	1.41		BET OLIVE ST & VINE ST	0 2		0	0	2956499	.118260	.18	.00	.645 .805	.42	
736	1.42	1.42		AT VINE ST	0 2	2		1	2956500	.029565	.24 .18	.00	.645	.00	
736	1.43	1.45		BET VINE ST & SPRINGDALE ST	02		0 2	0	2956499	.088695 .029565	.18	.00	. 645	.00	
736	1.46 1.47	1.46 1.48		AT SPRINGDALE ST BET SPRINGDALE ST & MELBA ST	0 2	2		0	2956500 2956499	.029565	.18	.00	.645	.00	
736 736	1.47	1.48		BET SPRINGDALE ST & MELBA ST AT MELBA ST	0 2	2		5	2956499 4142750	.029565	. 24	1.21	.739	1.63	
736	1.49	1.52		BET MELBA ST & PELHAM ST	0 2		0	0	5328999	.159870	.18	.00	. 558	.00	
136	1.50	1.32	33	DET MEDDA ST & PERRAM ST	J 2	2	U	U	J320373	.135670	. 10		, 555		



Queries Selected: Town(Trumbull), Date(Year:All or 1/1/2015 to 12/31/2016), Severity(All), Route Class(State), Road Number(), Mile Markers 0.26 to 0.38



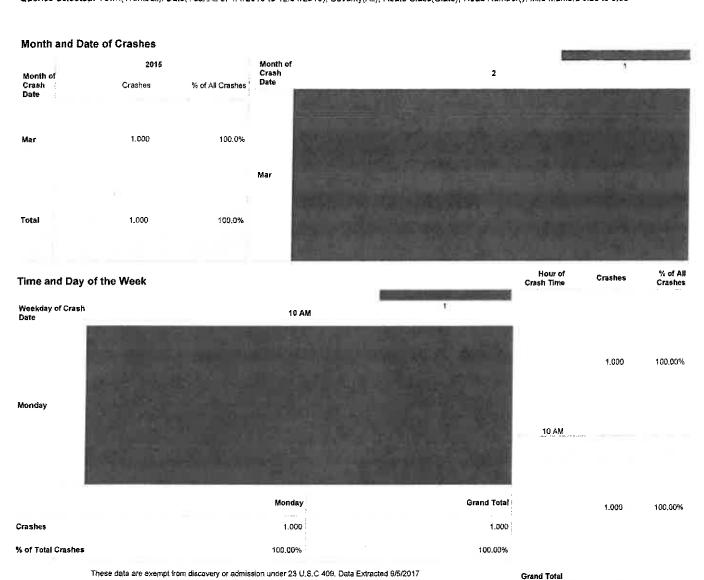
Injuries per Crash Fatalities per Crash



Injuries per Crash	Crashes	% of All Crashes	Fatalities per Crash	Crashes	% of All Crashes
0	1.000	100.00%	0	1,000	100.00%
Grand Total	1.000	100.00%	Grand Total	1.000	100.00%



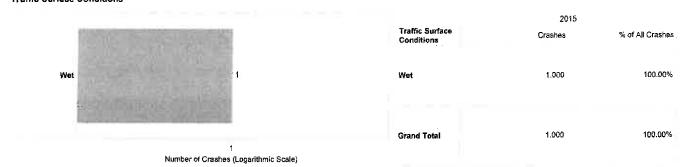
Queries Selected: Town(Trumbull). Date(Year:All or 1/1/2015 to 12/31/2016), Severity(All), Route Class(State), Road Number(), Mile Markers 0.26 to 0.38



Clessgraphy Caroling Constant Crash Conditions Residual R

Queries Selected: Town(Trumbull), Date(Year:All or 1/1/2015 to 12/31/2016), Severity(All), Route Class(State), Road Number(). Mile Markers 0.26 to 0.38

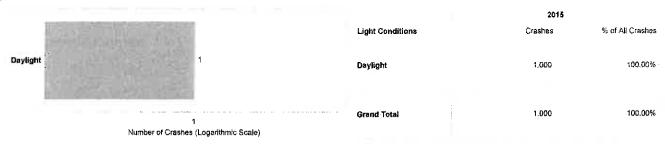
Traffic Surface Conditions



Weather Conditions

	2015	
Weather Conditions	Crashes % of All Crashes ;	
Clear	1,000 109,00% ;	Clear 2
Grand Total	1.000 100.00%	i Number of Crashes (Logarithmic Scale)

Light Conditions



These data are exempt from discovery or admission under 23 U.S.C 409, Data Extracted 9/5/2017

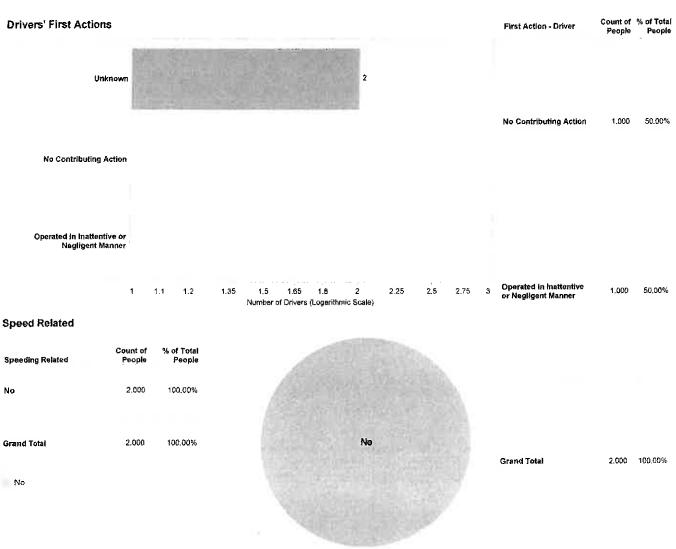
Fondway Contributing Contributing Crash Manner Figure Contributing and Location Figure 1 Figure Contribution Figure Contributi

Queries Selected: Town(Trumbull), Date(Year:All or 1/1/2015 to 12/31/2016), Severity(All), Route Class(State), Road Number(), Mile Markers 0.26 to 0.38

Location of 1st Harmful Event **Manner of Crashes** On Roadway Number of Crashes (Logarithmic Scale) Number of Crashes (Logarithmic Scale) Rear End Location Of First Harmful Event Crashes % of All Crashes 1.000 100.00% On Roadway Grand Total 1.000 100.00% Manner Of Crash % of All Crashes Crashes 100,00% Rear End 1,000 Grand Total 1.000 100.00%

Scaliscit State Ambaig Election Statum Driver Actions Driver - Petito trium Motorcy and Crashes - Crashes

Queries Selected: Town(Trumbull), Date(Year:All or 1/1/2015 to 12/31/2016), Severity(All), Route Class(State), Road Number(), Mile Markers 0.25 to 0.38



These data are exempt from discovery or admission under 23 U.S.C 409, Data Extracted 9/5/2017

CAPACITY ANALYSIS PROCEDURES

CAPACITY ANALYSIS PROCEDURES

Intersections – Four methods of analysis are needed to evaluate different kinds of intersections. These methods are based on procedures found in the Sixth Edition of the Highway Capacity Manual 2016 and are described below.

Two-Way STOP-Controlled Intersections (TWSC)

One typical configuration is a four-leg intersection, where the major street is uncontrolled, while the minor street is controlled by STOP signs. The other typical configuration is a three-leg intersection, where the single minor-street approach is controlled by a STOP sign.

Theoretical Basic – Gap-acceptance models begin with the recognition that TWSC Intersections give no positive indication or control to the driver on the minor street as to when it is appropriate to leave the stop line and enter the major street. The driver must determine when a gap on the major street is large enough to permit entry and when to enter, on the basis of the relative priority of the competing movements. This decision-making process has been formalized analytically into what is commonly known as gap-acceptance theory. Gap-acceptance theory includes three basic elements: the size and distribution (availability) of gaps on the major street, the usefulness of these gaps to the minor-street drivers, and the relative priority of the various movements at the intersection.

Critical Headway and Follow-Up Headway – The critical headway is defined as the minimum interval in the major street traffic stream that allows intersection entry for one minor-street vehicle. Thus, the driver's critical headway is the minimum headway that would be acceptable. Critical headway can be estimated on the basis of observations of the largest rejected and smallest accepted headway for a given intersection. The follow-up headway is defined as the time between the departure of one vehicle from the minor street and the departure of the next vehicle using the same major-street headway, under a condition of continuous queuing on the minor street.

Base Critical Headways for TWSC Intersections

VEHICLE MOVEMENT		BASE CRITICAL HEADW	AY
	Two Lanes	Four Lanes	Six Lanes
Left turn from major	4.1	4.1	5.3
U-turn from major	N/A	6.4 (wide) 6.9 (narrow)	5.6
Right turn from minor	6.2	6.9	7.1
Through traffic 0n	1-stage:6.5	1-stage:6.5	1-stage:6.5*
major	2-stage, stage I: 5.5	2-stage, stage I: 5.5	2-stage, stage I: 5.5*
	2-stage, Stage II: 5.5	2-stage, Stage II: 5.5	2-stage, Stage II: 5.5*
Left turn from	1-stage:7.1	1-stage:7.5	1-stage:6.4
minor	2-stage, stage I: 6.1	2-stage, stage I: 6.5	2-stage, stage 1: 7.3
	2-stage, Stage II: 6.1	2-stage, Stage II: 6.5	2-stage, Stage II: 6.7

^{*}Use caution; values estimated

Base Follow-up Headways for TWSC Intersections

	BAS	SE FOLLOW-UP HEADW	/AY
VEHICLE MOVEMENT	Two Lanes	Four Lanes	Six Lanes
Left turn from major	2.2	2.2	3.1
U-turn from major	N/A	2.5 (wide) 3.1 (narrow)	2.3
Right turn from minor	3.3	3.3	3.9
Through traffic on major	4.0	4.0	4.0
Left turn from minor	3.5	3.5	3.8

Level Of Service Criteria – LOS for a TWSC intersection is determined by the computed or measured control delay. For motor vehicles, LOS is determined for each minor-street movement (or shared movement) as well as major-street left turn. LOS is not defined for the intersection as a whole or for major-street approaches. LOS F is assigned to the movement if the volume-to-capacity ratio for the movement exceeds 1.0, regardless of the control delay.

Automobile Mode – The methodology applies to TWSC intersections with up to three lanes (either shared or exclusive) on the major-street approaches and up to three lanes on the minor-street

approaches (with no more than one exclusive lane for each movement on the minor-street approach). Effects from other intersections are accounted for only in situations in which a TWSC intersection is located on an urban street segment between coordinated signalized intersections. In this situation, the intersection can be analyzed by using the procedures in urban street segment.

Level-of Service Criteria for Automobile Mode

CONTROL DELAY		ME-TO-CAPACITY ATIO
(SECONDS PER VEHICLE)	1.0	>1.0
0- 10 >10 to 15 >15 to 25	A B C	F F F
>25 to 35 >35 to 50 >50	D E F	F F

Note: The LOS criteria apply to each lane on a given approach and to each approach on the minor street. LOS is not calculated for major-street approaches or for the intersection as a whole.

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CAPACITY ANALYSIS WORKSHEETS

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Build Conditions

Intersection						
Int Delay, s/veh	0.4					
•			NIDI	NIDT	CDT	SBR
Movement	EBL		NBL		SBT_	ODM
Lane Configurations	*/*		,	र्स	∱ ≱	А
Traffic Vol, veh/h	12				432	4
Future Vol, veh/h	12			441	432	4
Conflicting Peds, #/hr				0	0	0
Sign Control	Stop			Free	Free	Free
RT Channelized		140170		None	-	None
Storage Length	0			-	-	-
Veh in Median Storag	-		77.0	0	0	-
Grade, %	0				0	-
Peak Hour Factor	92				92	92
Heavy Vehicles, %	2		2	2	2	2
Mvmt Flow	13	13	4	479	470	4
Major/Minor	Minor2		Major1	٨	Major2	
Conflicting Flow All	959		474	0	3	0
Stage 1	472	-T12	717		- 5	-
Stage 2	487	-	-		32	
Critical Hdwy	6.42	6.22	4.12	_	(5)	X20
Critical Hdwy Stg 1	5.42	Ų.ZZ	7.12	-	_	
Critical Hdwy Stg 2	5.42	-	-	::: ::::::::::::::::::::::::::::::::::	_	(5)
		3.318	2 210	65 52	2	
Follow-up Hdwy	285	592	1088	25	<u>.</u>	200
Pot Cap-1 Maneuver	628	382	1000	~	#X	(20)
Stage 1		-	950		**	000
Stage 2	618	-		-	*) 03	300
Platoon blocked, %	004	E00	4000		7.0	(41
Mov Cap-1 Maneuver	284	592	1088	~	52	
Mov Cap-2 Maneuver	284	-	۰	-		
Stage 1	625	20	170	2	T _C	
Stage 2	618	-	***	3	*	
Approach	EB		NB		SB	
HCM Control Delay, s	15.1		0.1		0	
HCM LOS	С					
	-					
Minor Lane/Major Mym	ıt	NBL	NBT E	Rin1	SBT	SBR
Minor Lane/Major Mym	IL.				וטט	טטת
Capacity (veh/h)		1088	-	384	-	1.0
HCM Lane V/C Ratio		0.004	_	0.068	()	**
HCM Control Delay (s)		8.3	0	15.1	1.35)(*
HCM Lane LOS		A	Α	С	to acco	25
HCM 95th %tile Q(veh)		0	-	0.2	. *0	-

0.3	3		-		•
EBL	. EBF	NBL	. NBT	SBT	SBR
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Ciop					None
٥				12	NOTIC
				0	-
		•			-
					-
					92
					2
7	9	14	695	595	12
Minor2		Major1	٨	/laior2	
					0
		/טט			
		-	-		-
		-	5	55	350
	6.22	4.12	2.00	Ŧ.	2
	15	-	25	-	-
	-	-	*	•	
3.518	3.318	2.218	⊕		9
172	500	971	*	-	-
547	-	-			59
	-	_	*		92
			2	2.0	82
160	E00	071			72
			8		32
	-		-		-
	-)*	-	•	-
481		(5)	50	00	12
EB		NB		SB	
					•
		V.E		V	
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nt	NBL	NBT E		SBT	SBR
		-	271	12	*
	971	-			
	971 0.015		0.056	3.5	
	0.015		0.056	:: :*	
	0.015 8.8	0	0.056 19.1	Ş÷.	8
	0.015	-	0.056	35 35 35 34	
	EBL 6 6 6 7 0 0 92 2 7 7 Minor2 1324 601 723 6.42 5.42 5.42 5.42 5.42 5.42 5.42 5.42 5	6 8 6 8 6 8 7 0 0 0 5 8 8 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9 9	EBL EBR NBL 6 8 13 6 8 13 7 0 0 0 8top Stop Free - None 9e, # 0 92 92 92 2 2 2 7 9 14 Minor2 Major1 1324 601 607 601 723 6.42 6.22 4.12 5.42 5.42 3.518 3.318 2.218 172 500 971 547 481 168 500 971 168 168 500 971 168 168 500 971 168 168 534 481 EB NB 19.1 0.2 C	EBL EBR NBL NBT 6 8 13 639 6 8 13 639 0 0 0 0 0 Stop Stop Free Free - None - None 0 0 92 92 92 92 2 2 2 2 2 7 9 14 695 Minor2 Major1 M 1324 601 607 0 601 723 723 6.42 6.22 4.12 5.42 3.518 3.318 2.218 172 500 971 547 481 168 500 971 168 168 500 971 168 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534 168 534	EBL EBR NBL NBT SBT 6 8 13 639 547 6 8 13 639 547 0 0 0 0 0 Stop Free Free Free Free - None - None - 0 - - 0 0 92 92 92 92 92 2 2 2 2 2 2 7 9 14 695 595 Minor2 Major1 Major2 Major2 1324 601 607 0 - 601 - - - - 723 - - - - 6.42 6.22 4.12 - - 5.42 - - - - 481 - - - 168 500<